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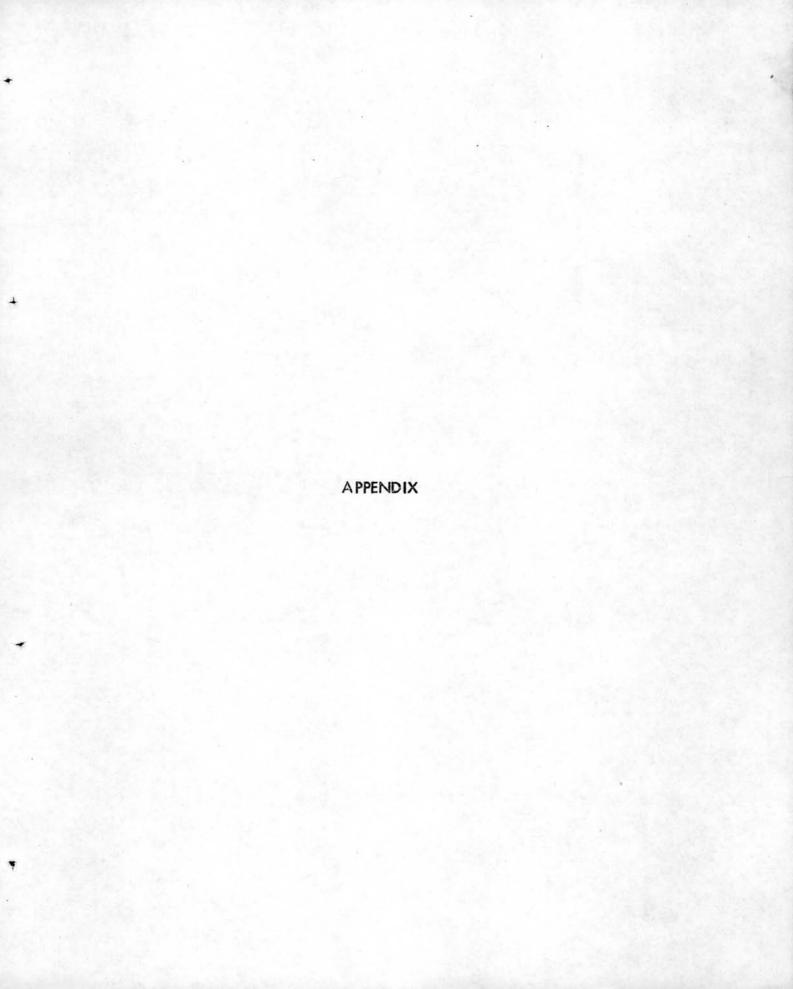
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APPENDIX A

Table A 1 Calculation of Equivalence Factors for Heavy Vehicles

		- axle				3 - axle	CANAL STATE OF THE	
Weight, tonnes	0.35wt.	0.65wt.	Eq.factor	Weight, tonnes	0.20wt.	0.41wt.	0.39wt.	Eq.facto
5.5	1.925	3.575	0.025	7.2	1.44	2.952	2.808	0.019
7.0	2.450	4.550	0,075	12.0	2.40	4.920	4.680	0.185
7.2	2.520	4.680	0.085	13.0	2.60	5.330	5.070	0.265
7.7	2.695	5.005	0.115	14.0	2.480	5.740	5.460	0.369
7.8	2.730	5.070	0.122	15.0	3.00	6.150	5.850	0.504
7.9	2.765	5.135	0.129	16.0	3.20	6.560	6.240	0.673
8.0	2.800	5.200	0.137	16.2	3.24	6.642	6.318	0.712
8.6	3.010	5.590	0.189	16.5	3.30	6.765	6.435	0.773
9.0	3.150	5.850	0.232	16.7	3.34	6.847	6.513	0.815
9.4	3.290	6.110	0.283	17.0	3.40	6.970	6.630	0.885
9.5	3.325	6.175	0.296	17.6	3.52	7.216	6.864	1.034
9.6	3.360	6.240	0.311	17.8	3.56	7.298	6.942	1.088
9.7	3.395	6.305	0.325	18.0	3.60	7.380	7.020	1.144
9.8	3.430	6.370	0.341	18.2	3.64	7.462	7.098	1.202
10.0	3.500	6.500	0.373	18.4	3.68	7.544	7.176	1.263
10.3	3.605	6.695	0.426	18.6	3.72	7.626	7.254	1.326
10.5	3.675	6.825	0.465	19.0	3.80	7.790	7.410	1.459
11.0	3.850	7.150	0.573	19.2	3.84	7.872	7.488	1,530
11.2	3.920	7.280	0.621	19.5	3.90	7.995	7.605	1.642
11.3	3.955	7.345	0.647	19.8	3.96	8.118	7.722	1.757
11.4	3.990	7.410	0.673	20.0	4.00	8.200	7.800	1.838
11.5	4.025	7.475	0.700	20.2	4.04	8.282	7.878	1,922
11.7	4.095	7.605	0.756	20.3	4.06	8.323	7.917	1.965
12.0	4.200	7.800	0.848	20.5	4.10	8.405	7.995	2.054
				20.8	4.16	8.528	8.112	2.193

 1 Equivalence factor = $\Sigma_{i} \left(\frac{L_{i}}{2200} \right)^{4.5}$

where $L_i = load$ on axle i, kg



Table A 2 Observations and Summed Equivalence Factors and Average Payload,
Heavy Vehicles Southbound on Saraburi - Ban Phu Kae Segment

	2 -	axle			3	- axle	
Weight, tonnes	Eq.factor	Frequency	Total Eq.factor	Weight, tonnes	Eq.factor	Frequency	Total Eq.facto
7.0	0.075	2	0.150	15.0	0.504	1	0.504
8.0	0.137	1	0.137	16.0	0.673	3	2.019
9.6	0.311	1	0.311	16.7	0.816	1	0.816
9.8	0.341	1	0.341	17.0	0.885	5	4.425
10.0	0.373	7	2.611	17.6	1.034	4	4.136
10.3	0.426	1	0.426	17.8	1.088	9	9.792
10.5	0.465	4	1.860	18.0	1.144	68	77.792
11.0	0.573	3	1.719	18.2	1.202	3	3.606
11.2	0.621	1	0.621	18.4	1.263	1	1.263
11.3	0.647	1	0.647	18.6	1.326	1	1.326
11.4	0.673	2	1.346	19.0	1.459	2	2.918
11.5	0.700	1	0.700	19.2	1.530	1	1,530
12.0	0.848	1	0.848	19.5	1.642	6	9.852
-:				19.8	1.757	3	5,271
Total		26	11.717	20.0	1.838	21	38.598
		11 7	17	20.2	1.922	1	1.922
Av. equ	ivalence fa	$ctor = \frac{11.7}{26}$	= 0.45	20.3	1.965	1	1.965
		20		20.5	2.054	5	10,270
Av. unla	den weigh	t = 5.5 tor	nes	20.8	2.193	1	2.193
Av. pay	load = 265.	$\frac{5}{5}$ - 5.5 = 4	.7 tonnes	Total		137	180.198

Av. equivalence factor = $\frac{180.198}{137}$ = 1.315

Av. unladen weight = 7.2 tonnes

Av. payload = $\frac{2528.3}{137}$ - 7.2 = 11.3 tonnes

Table A3 Observations and Summed Equivalence Factors and Average Payload.

Heavy Vehicles Northbound on Saraburi - Ban Phu Kae Segment

	2	- axle			3	- axle	
Weight, tonnes	Eq.factor	Frequency	Total Eq.factor	Weight, tonnes	Eq.factor	Frequency	Total Eq.factor
5.5	0.025	3	0,200	7.2	0.019	110	2.090
7.2	0.085	1	0.085	12.0	0.185	1	0.185
7.7	0.115	1	0.115	13.0	0.265	1	0.265
7.8	0.122	1	0.122	14.0	0.369	1	0.369
7.9	0.129	1	0.129	15.0	0.504	2	1.008
0.8	0.137	2	0.274	16.2	0.712	1	0.712
6.3	0.189	1	0.189	16.5	0.773	2	1.545
9.4	0.283	1	0.283	17.0	0.885	6	5.310
9.5	0.296	1	0.296	17.6	1.034	5	5.170
9.7	0.325	1	0.325	17.8	1.088	1	1.083
9.8	0.341	1	0.341	18.0	1.144	5	5.720
10.0	0.373	4	1.492	19.0	1,459	1	1.459
10.5	0.465	1	0.465	20.0	1.838	1	1.838
11.0	0.573 0.756	1	0.573 0.756	Total		137	26.760
Total	3.730	26	5.645	Av. equi	valence fac	$ctor = \frac{26.76}{137}$	0 = 0, 195

Av. equivalence factor $=\frac{5.645}{26} = 0.217$ Av. payload $=\frac{1247}{137} = 7.2 = 1.9$ ton: es

Table A4 Observations and Summed Equivalence Factors and Average Payload.

Hagyy Vahialas Southbound on Ban Dh

		-axle			3	- axle	
Weight, tonnes	Eq.factor	Frequency	Total Eq.factor	Weight, tonnes	Eq.factor	Frequency	Total Eq. factor
7.0	0.075	1	0.075	16:0	0.673	2	1,345
9.6	0.311	1	0.311	17.0	0.885	3	2.655
9.8	0.341	1	0.341	17.6	1.034	1	1.034
10.0	0.373	6	2,238	17.8	1.088	5	5,440
10,5	0.465	1	0.465	18.0	1.144	32	36,609
11.0	0.573	2	1.146	18.4	1.263	1	1.263
11.5	0.700	1	0.700	18.6	1.326	1	1,325
Total		13	5.276	19.0	1.459	1	1.459
10101		10	3.270	19.2	1.530	1	1,530
				19.5	1.642	3	4.925
				19.8	1.757	2	3,514
				20.0	1.838	5	9,190
				20.2	1,922	1	1,922
				20.3	1.965	1	1,965
				20.5	2.054	3	6.162
				Tota!		62	78,418

2 -axle	3 - axle
	Av. equivalence factor = $\frac{78.418}{62}$ = 1.265
Av. payload = $\frac{130.4}{13}$ - 5.5 = 4.5 tonnes	Av. payload = $\frac{1140.9}{62}$ - 7.2 = 11.2 tonnes

Table A.5 Observations and Summed Equivalence Factors and Average Payload,
Heavy Vehicles Northbound on Ban Phu Kae - Lamnarai Segment

		2 - axle		3 - axle				
Weight, tonnes	Eq.factor	Frequency	Total Eq.factor	Weight, tonnes	Eq.factor	Frequency	Total Eq.factor	
5.5	0.025	11	0.275	7.2	0.019	52	0.988	
10.0	0.373	1	0.373	12.0	0.185	1	0.185	
11.7	0.756	1	0.756	15.0	0.504	2	1.008	
				16.2	0.712	1	0.712	
Total		13	1.404	16.5	0.773	2	1.544	
		***************************************		17.0	0.885	2	1.770	
				18.0	1.144	1	1.144	
				20.0	1.838	1	1.838	
				Total		62	9,191	

Av. equivalence factor = $\frac{1.404}{13}$ = 0.108 Av. equivalence factor = $\frac{9.191}{62}$ = 0.148 Av. payload = $\frac{82.2}{13}$ = 5.5 = 0.8 tonnes Av. payload = $\frac{537.6}{62}$ - 7.2 = 1.5 tonnes

Eq. factor = 0.232 for each direction.

Table A 6 Calculations of Exponential Regression Curve for Traffic Volume:

Saraburi - Ban Phu Khae Station 121 + 000.

year	×	(APT)	×2	logy	xlogy
1969	1	4,129	1	3,61584	3.61584
1970	2	5,164	4	3.71299	7.42597
1971	3	5,153	9	3.71206	11.13618
1972	4	5,239	16	3.71925	14.87699
1973	5	5,872	25	3.76879	18.84393
1974	6	5,398	36	3.73223	22.39340
Total	21	30,955	91	22,26116	78,29231

$$\bar{x} = 3.5$$
, $\log y = 3.71019$

Exponential Regression
$$b = \frac{n\Sigma x \log y - (\Sigma x)(\Sigma \log y)}{n\Sigma x^2 - (\Sigma x)^2}$$

 $= \frac{6(78,29231) - (21)(22.26116)}{6(91) - (21)^2} = 0.021614$
 $a = \log y - b\bar{x}$ $c = 10^a$ $d = 10^b$
 $= 3.71019 - 0.2161(3.5)$ $= 10^{3.63454}$ $= 10^{0.021614}$
 $= 3.63454$ $= 4310.6$ $= 1.05103$

$$y = cd^{x}$$
; $y = (4310.6)(1.051)^{x}$

Table A 7 1970 ADT from Regression, and Percentage Distribution of type of vehicles obtained from Thai Highway Department:

Saraburi - Ban Phu Kae Segment.

Vehicle Type	Percentage at total vehicles	1970 ADT
Carsand taxis	33.4	1590
Light trucks	9.4	448
Light buses	7.7	367
Heavy buses	9.4	448
2 - axle trucks	14.7	700
3 – axle trucks	25.4	1209
Total	100	4762

Table A 8 Calculations of Exponential Regression Curve for Traffic Volume:

Ban Phu Khae - Lamnarai Station 20 + 900

year	×	ADT	×2	logy	xlogy
1972	1	1229	1	3.1136	3,1136
1973	2	1393	4	3.1440	6.2879
1974	3	1224	9	3.0878	9,2633
1975	4	1473	16	3.1682	12.6728
Total	10	5389	30	12,5135	31,3377

$$\bar{x} = 2.5$$
, $\bar{\log}y = 3.1284$

Exponential Regression b =
$$\frac{n\Sigma x \log y - (\Sigma x) (\Sigma \log y)}{n\Sigma x^2 - (\Sigma x)^2}$$

= $\frac{4(31.3377) - 10(12.5135)}{4(30) - (10)^2}$ = 0.01079
a = $\overline{\log y} - b\overline{x}$ | c = 10^a | d = 10^b
= 3.1284 - (0.01079)(2.5) | = $10^{3.10143}$ | = $10^{0.01072}$
= 3.10143 | = 1263 | = 1.025

$$y = cd^{x}$$
, $y = (1263)(1.025)^{x}$

Table A 9 1970 ADT from Regression, and Percentage Distribution of type of vehicles obtained from Thai Highway Department:

Ban Phu Kae - Lamnarai Segment.

Vehicle Type	Percentage at total vehicles	1970 ADT		
Cars and taxis	22.2	273		
Light trucks	13.3	164		
Light buses	9.6	118		
Heavy buses	8.7	107		
2 - axle trucks	16.2	200		
3 – axle trucks	30.0	370		
Total	100.0	1232		

APPENDIX B

Table B 1

The Thai Highway Department classification of maintenance cost accounts is set forth in the following listing.

- 1) Administration
 - Clerical administration
 - Cost of services, office expenses, etc.
 - Routine field inspection
 - Unkeep of District workshops
- 2) Land and buildings
 - Maintenance of land and buildings
- 3) Routine surface operations
 - Heavy grading
 - Light grading
 - Soil aggregate surface patching
 - Skin patching of paved roads
 - Minor repair of asphalt pavement
 - Concrete patching
 - Concrete joint sealing
 - Roadway clearing
- 4) Shoulders, side road approaches, and median strips
 - Shoulder grass cutting
 - Shoulder patching
 - Shoulder light grading

5) Drainage

- Drainage cleaning
- Waterway restoration
- Culvert cleaning and repairing

6) Roadside maintenance

- Side slope repairing
- Roadside grass cutting
- Maintenance of roadside development

7) Traffic service operations

- Traffic marking
- Maintenance of road signs, guide posts, guard rails, right of way posts, and km. posts
- Maintenance of traffic signals and road lighting

8) Maintenance and repair of structures

- Bridge maintenance and repair
- Miscellaneous structures

9) Emergencies

- Wash outs
- Major landslides
- Major accidents
- Miscellaneous disasters

10) Special maintenance

- Asphalt seal coating
- Asphalt surface leveling or redressing

- Surface regravelling
- Major repair of asphalt pavement
- Major repair of concrete pavement
- Major repair of shoulders, side road approaches and median strips
- Major side slope repairing
- Dust palliatives

11) Betterments

- Improvement of geometry
- Pavement widening
- Shoulder widening or improvement
- Base stabilization
- Asphalt surfacing
- Upgrading of paved roads
- Replacement or extension of minor structures
- Construction of minor retaining walls
- Elimination of flooding and improvement of drainage
- Erosion protection
- Paving of ditches
- Provision of rest areas
- Provision of bus stops and shelters
- Installation of traffic signals and road lighting
- Installation of guard rail
- Installation of road signs, guide posts, right of way posts, and km. posts
- Roadside clearing
- Planting and landscaping

Table B2

The main factors that affect vehicle running costs may be grouped as follows:

- 1) The highway
 - Distance
 - Geometric design, transverse and longitudinal
 - Character of roadway surface
 - Traffic volume, composition, traffic controls, and speed changes
 - Legal restraints
- 2) The vehicle
 - Road weight and weight-horsepower ratio
 - Engine design
 - Transmission and rear-axle ratios
 - Tyre size and tyre pressure
 - Vehicle dimensions and vehicle dynamic characteristics
 - Mechanical condition of engine, power transmission, and braking systems
 - Type of fuel
- 3) The operator
 - Rates of acceleration and deceleration (speed changes)
 - Number and range of speed changes
 - Number and timing of gear changes
 - Cruising speed
 - Character of use, trip length, and annual mileage
 - Care of vehicle
- 4) The weather and topography
 - Air temperature, air pressure, and air humidity
 - Wind direction and velocity
 - Rain, snow, and ice conditions on roadway
 - Altitude and topography

From: WINFREY, R. (1969)

APPENDIX C
SUMMARY OF MAINTENANCE AND VEHICLE OPERATING COSTS

Table C1 Maintenance Cost and Vehicle Operating Costs at 5 Percent Traffic Growth

Analysis Year		Sec	Maintena tion	ince Costs, Ba	ht	Vehicle Operating Costs, Baht Section					
	1	2	3	4	Total	1	2	3	4	Total	
4 5 6 7 8 9 10 11	979 9,050 13,912 ₁ 91,260 979 14,224 93,397 1,522 18,303	15, 134 15, 134 59, 058 133, 916 139, 854 146, 088 631, 607 ² 115, 134 31, 574	16,810 16,810 16,810 16,810 102,883 150,255 999,2742 16,810 16,810	21,645 21,645 21,645 21,645 115,006 166,389 1,087,342 ² 21,645 21,645	54,568 62,639 111,435 263,631 358,721 476,956 2,811,620 55,111 86,332	3,809,551 4,065,942 4,349,162 4,651,366 4,971,760 5,312,969 5,676,390 6,065,553 6,483,638	36, 272, 332 38, 436, 205 40, 799, 708 43, 299, 782 45, 871, 347 48, 550, 578 51, 371, 282 54, 381, 706 57, 616, 761	32,452,828 34,343,850 36,373,745 38,486,714 40,643,457 42,876,745 45,214,854 47,676,942 50,278,126	35, 181, 463 37, 231, 642 39, 432, 405 41,723, 227 44, 061, 475 46, 482, 694 49, 017, 549 51, 686, 824 54, 506, 905	107,716,173 114,077,639 120,955,020 128,161,089 135,548,039 143,222,986 151,280,075 159,811,025 168,885,430	
				Year 4	ADT 12 ADT	4,762 ³ 7,036 ³ 0.8	2,381 ³ 3,518 ³ 14.1	1,232 1,820 24.8	1,232 1,320 26.9		

¹Surface dressing required at this time because cumulative traffic loading has exceeded 1.5 x 10⁵ equivalent standard axles

²Surface dressing required because 7 years was specified for this study as the maximum duration to first surface dressing treatment

The actual highway volumes on Section 1 and Section 2 are equal. There shown for Section 2 are half of the Section 1 volume as only a 2-lane carriageway was built as new road in Section 2; the old 2-lane carriageway remained in service for northbound traffic.

Table C 2 Maintenance Costs and Vehicle Operating Costs at 10 Percent Traffic Growth

Analysis		0		ce Costs, Bo	ht			rating Costs,	Baht			
Year		Sectio	ก				Sectio	n				
	1	2	3	4	Total	1	2	3	4	Total		
4.	1,116	17,532	17,600	24,629	60,877	3,809,551	36, 272, 332	32,452,828	35, 181, 462	107,716,173		
5	9,723	17,532	17,600	24,629	69,484	4,260,599	40, 282, 086	35,970,302	38,994,848	119,507,835		
3	15,308,	77,623	17,600	24,629	135,160	4,754,256	44, 626, 946	39,733,717	43,074,307	132, 189, 726		
7	93,427	154, 101	17,600	24,629	289,757		49,413,362	43, 842, 164	47, 528, 801	146,087,728		
8	2,326	167,755	164,249	183,694	. 518,024		54,709,720	48, 362, 584	52,429,418	161,431,825		
9	19,998	661,747	185,986	207, 2712	1,075,002	6,642,859	60,626,797	53, 385, 711	57,875,052	178,530,419		
10	98,586	17,532	1,045,169	1, 139, 250		7,457,219	67, 294, 049	58,997,900	63,959,346	197,708,514		
11	8,001	119,709	17,600	24,629		8,390,797	74, 894, 258	65, 280, 440	70,770,440	219, 335, 935		
12	102,943	237,462	17,500	24,629	382,634	9,462,949	83,484,959	72,316,596	78, 398, 622	243, 663, 126		
				Year	4 ADT	4,7623	2,3813	1,232	1,232			
				Year	12 ADT	10,2083	5, 104 ³	2,641	2,641			
				Lengtl	h, km.	0.8	14.1	24.8	26.9			

^{1, 2, 3} See foot note Table C 1

Table C 3 Maintenance Costs and Vehicle Operating Costs at 15 Percent Traffic Growth

Analysis		Maintenance Costs, Baht					Vehicle Operating Costs, Baht			
Year		Secti	on				Section			
	1	2	3	4	Total	1	2	3	4	Total
4	1,116	17,532	17,500	24, 529	60,877	3,809,551	36, 272, 332	32,452,828	35, 161, 462	107,716,173
5	10,358	17,532	17,600	24,629		4,455,292		37,597,001	40,758,323	124, 938, 817
6	16,625	94,295	17,600	24,629	153, 149	5, 179, 345	48,641,666	43, 258, 097	46, 895, 441	143, 874, 549
7	95,65T	173,580	48,032	57,636		6,022,015		49,727,911	53,909,256	165,794,683
8	5,662	196,984,		222,978	626,091	7,036,993	64, 887, 337	57, 241, 301	62,054,438	191, 220, 069
9	101,394	702,870	227, 893,	252,727	1,284,884	8,254,757	75, 189, 030	66,031,722	71,584,122	221,059,631
10	12,268	17,532	1,101,784	1,200,6554	2,332,236	9,725,168	87,482,890	76,351,636	82,772,053	256, 331, 747
11	108,991	248, 235	17,600	24,629	339,939	11,507,562	102, 130, 363	88,481,529	95, 922, 347	298,041,901
12	21,003	810,349	123,886	139,913	1,095,151		119,570,833	102,789,479	111, 434, 122	347,469,401
				Year 4	4 ADT	4,7623	2,381 ³	1,232	1,232	
				Year 1	2 ADT	14,5673	7,2843	3,769	3,739	
				Length	, km.	0.8	14.1	24.8	26.9	

^{1, 2, 3&}lt;sub>See</sub> foot note Table C 1

Table C 4 Discounted Maintenance and Vehicle Operating Costs at 5 Percent Fraffic Growth, Baht

Analysis Maintenance		Discounted at			Vehicle	Discounted at			
Year	cost	10%	12%	15%	operating cost	10%	12%	15%	
4	5 4, 5 68	37,271	34,679	31, 199	107,716,173	73,571,596	68, 455, 576	41,587,072	
5:	62,639	38,894	35,543	31, 143	114,077,639	70, 833, 239	64,730,717	56,716,748	
6	111,435	62,902	55,456	48,176	120,955,020	68,275,95 6	61, 279, 579	52, 292, 193	
7	263,631	135, 284	119,253	99, 109	128, 161, 089	65,766,903	57,973,569	48, 180, 500	
8	358,721	167,346	144,881	117,267	135,548,039	63, 234, 161	54,745,581	44,310,894	
9	475,956	202, 276	171,995	135,581	143, 222, 986	60,740,527	51, 547, 546		
10	2,811,620	1,084,001	905,266	694,939	151, 200, 075	58, 325, 018	48,700,136		
11	55,111	19,316	15,843	11,843	159,811,025	56,012,789	45,941,852		
12	88,332	28, 145	22,673	16,510	168, 885, 430	53, 812, 103	43, 348, 685		
Total		1,775,435	1,506,589	1, 185, 820		570, 572, 292	495, 831, 341	407, 110, 630	

Table C 5 Discounted Maintenance and Vehicle Operating Costs at 10 Percent Traffic Growth, Baht

Analysis Maintena		D	iscounted	at	Vehicle	Discounted at			
Year	cost	10%	12%	15%	operating cost	10%	12%	15%	
4	60,877	41,500	3 8 0, 688	34,807	107,715,173	73,571,596	68,455,575	61,587,072	
5	69,484	43, 144		34,546	119,507,835		67,811,955	59,416,515	
6	135,160	76,294		58,433	132, 189, 726	74,617,654	66,971,429	57, 149, 260	
7	209,757	148,694	131,071	108,930	146,087,728	74,955,104	66,002,669	54,919,78	
8	510,024	241,562	209, 221	169,343	161,431,825	75, 309, 138	65, 199, 607	52,772,350	
9	1,075,002	455,903	307,656	305,583	178, 530, 419	75,714,326	64, 379, 859	50,749,488	
10	2,300,537	886,957	740,711	568,658	197,708,514	76, 225, 191	63, 656, 850	48,870,52	
11	169,939	59,563		36,527	219, 335, 935	76,875,907	63,053,840	47, 144, 773	
12	382,634	121,919		71,517	243, 553, 126	77, 638, 581	62, 542, 255	45,542,300	
Total		2,075,719	1,762,316	1,388,344		679, 123, 460	588, 154, 039	478, 152, 15	

Table C & Discounted Maintenance and Vehicle Operating Costs at 15 Percent Traffic Growth, Baht

Analysis	Maintenance	Discounted at			Vehicle	Discounted at		
Year	cost	10%	12%	15%	operating cost	10%	12%	15%
4	60,877	41,500	3 8, 688	34, 807	107,716,173	73,571,596	68,455,575	61,587,072
5	70,119	43,538		34,862	124,935,817	77,577,176	70,893,640	
- 3	153,149	06,449		66, 211	143, 874, 549	81, 213, 432	72,891,324	
7	374,899	192,382		140,938	165,794,683	85,078,888	74,997,095	
8	525,091	292,076		204,670	191,220,069	89, 205, 573	77,230,579	
9	1,284,884	544,916	463,342	365,244	221,059,631		79,716,319	
10	2,332,236	899, 178		576,493	256, 331, 747	98, 826, 985	82,531,952	
11	339,939	119, 147		73,068	298,041,901	104,461,868	85,679,925	
12	1,095,151	348,949		204,692	347,469,401	110,714,459	89, 186, 741	
Total		2,568,215	2,171,600	1,700,985		814, 400, 840	701, 583, 160	565,950,060

APPENDIX D

SUMMARY OF DATA

Table D1 Derivation of Engineering Costs for Saraburi - Lomsak Highway
Section 1 (sta. 1+800 to sta. 2+600 = 0.8 km)

- 1) Horizontal alignment: 0,02 degree /km. measured from the construction plans.
- Vertical alignment: intersection points and curve lengths as given in the construction plans.
- 3) Geometric design standards: 5 percent maximum gradient, design speed of 100 km/hr, and 60 meters minimum length of vertical curve.
- 4) Ground data: measured from the construction plans in terms of elevation at 100 meter chainage intervals.
- 5) Roadway cross section: roadway width 21.0 meters with crossfall of 1:50 (rise:run); shoulder width of 3.0 meters with crossfall of 1:50 determined from typical cross section. Other cross-section details were estimated average values: cut slope 1:2.5, fill slope 1:2.5, ditch depth 1.0 meter, ditch side slope 1:4. The width of the strip to be cleared of ground cover was 60 meters (the full width of right of way).
- 6) Soil characteristics: cut material was generally suitable for use as embankment. Earthwork bulking and compaction factors were 1.25 and 0.8, respectively; subgrade CBR was taken to be 5 percent.
- 7): Earthwork excavation: a unit cost of 24.53 Baht per m³ was used based on a weighted average of roadway excavation of earth, weathered rock, and rock.

- 8) Earthwork filling: a unit cost of 17.50 Baht per m³ was used. (In Thailand earthwork fills or embankments are paid for at unit costs which include fill, haulage, borrow, and spoil.)
- 9) Earthwork, haulage: zero cost.
- 10) Earthwork, borrow: zero cost.
- 11) Earthwork, spoil: zero cost.
- 12) No retaining walls.
- 13) Site slearance: 100 percent open country at unit cost of 5600 Baht per he.
- 14) Pavement: 4 layers from typical roadway cross section.
 - (a) 113 mm of selected subgrade (based on weighted average of total of study section) with a unit cost of 34.0 Baht per m³ from final report.
 - (b) 200 mm of sub-base (special) with estimated CBR of 20 percent at a unit cost of 150.0 Baht per m³ from final report.
 - (c) 150 mm of crushed rock base with estimated CBR of 80 percent at a unit cost of 132.0 Baht per m³ from final report.
 - (d) 50 mm of asphaltic concrete pavement at a unit cost of 520.6 Baht per m³ from final report.
- 15) Shoulders: 3 layers from typical roadway cross section.
 - (a) 113 mm of selected subgrade with a unit cost of 34.0 Baht per m³.
 - (b) 200 mm of sub-base with a unit cost of 50.0 Baht per m³.
 - (c) 200 mm of soil aggregate shoulder with a unit cost of 60.0 Baht per m³.
- 16) Rainfall: average annual rainfall of 1457 mm and maximum hourly rainfall of 79 mm from the final report.

- 17) Culverts call reinforced concrete pipe.
 - (a) 0.50 m diameter at a unit cost of 290.0 Baht per meter length.
 - (b) 0.60 m diameter at a unit cost of 355.0 Baht per meter length.
 - (c) 0.80 m diameter at a unit cost of 600.0 Baht per meter length.
 - (d) 1.00 m diameter at a unit cost of 900,0 Baht per meter length.
 - (e) 1.20 m diameter at a unit cost of 1300.0 Baht per meter length.
- 18) Average length of pipe is zero (no reinforced-concrete culverts required in Section 1).
- 19) Headwalls: unit cost of 1200.0 Baht per m³.
- 20) No cross-flow drainage.
- 21) No minor river crossings.
- 22) No major river crossings, but allow an additional cost of drainage of 394,056.0 Baht.
- 23) Miscellaneous costs are 709,457.0 Baht based on a proportion of total.
- 24) Overheads: zero.

Section 2 (sta. 2+600 to sta. 16+700 = 14.1 km)

- 1) Horizontal alignment: 2.38 degree/km measured from the construction plans, 2-4) As for Section 1.
- 5) Roadway cross section: roadway width 7.5 meters with crossfall of 1:50, shoulder width of 2.0 meters with crossfall of 1:25 determined from typical cross section. Other cross section details were estimated, average value: cut slope 1:2.5, fill slope 1:2.5, ditch depth 1.0 meter, ditch side slope 1:4. The width of strip to be cleared of ground cover was 60 meters measured from the construction plans.

- 6-13) As for Section 1.
- 14) Pavement: 4 layers from typical roadway cross section.
 - (a) As for Section 1.
 - (b) 200 mm of sub-base with estimated CBR of 20 percent at a unit cost of 50.0 Baht per m³ from final report.

(c-d) As for Section 1.

15-17) As for Section 1.

- 18) Average length of pipe is 15.29 meters from construction plans.
- 19) As for Section 1.
- 20) Cross-flow drainage: the size most commonly used is 1,20 meters diameter at an average spacing of 671 meters from the construction plans,
- 21) No minor river crossings.
- 22) Major river crossings at:

 sta. 6+981 RC.Bridge length 30.50 m (3-10.00 spans) x 11.00m; skew 0°

 sta. 15+206 RC.Bridge length 30.50 m (3-10.00 spans) x 11.00m; skew 0°

 Costs estimated to be 1,240,496 Baht, from the bills of quantities.
- 23) Miscellaneous costs are 594,565 Baht based on a proportion of total.
- 24) Overheads: zero.

Section 3 (sta. 16+700 to sta. 41+500 = 24.8 km)

- 1) Horizontal alignment: 11.88 degree/km measured from the construction plans.
- 2) As for Section 1.
- 3) Geometric design standards: 8 percent maximum gradient, design speed of 60 km/hr, and 60 meters minimum length of vertical curve.
- 4) As for Section 1.

5) As for Section 2: except cut slope is 1:0.8 (rise:run)

6-13) As for Section 1.

14) As for Section 2.

15-17) As for Section 1.

- 18) Average length of pipe is 23.57 meters from construction plans.
- 19) As for Section 1.
- 20) No cross-flow drainage.

21) Minor river crossings at:

Station	Catchment area, ha	Terrain run-off coeff.
17+662	46.0	0.8
18+733	5.0	0.8
19+002	12.0	0.8
19+747	21.0	0.8
25+500	17,0	0.8
26+250	21.0	0.8
28+150	30.0	0.8
28+275	28.0	0.8
30+200	40.0	0.8
34+460	15.0	0.8

22) Major river crossings at:

	Station	Length	Spans	Width	Skew
RC. Bridge	17+303	40.50	4-10.00	9.00	o° .
RC. Box culvert	19+214	width = 5.7	5; 2 boxes 2	.50×2.00;	length = 59.0; skew = 0°
RC. Bridge	20+177	30.532	3-10.00	9.00	20°
RC. Bridge	22+315	30.500	3-10.00	9.00	o°

	Station	Length	Spans	Width	Skew
RC. Bridge	23+179	24.532	3-8.00	9.00	20°
RC. Box culvert	24+514	width = 10.	132; 3 boxes	2.50×2.00	; length = 30.0; skew = 35
RC. Bridge	26+025	40.578	4-10.00	9.00	30°
RC. Bridge	29+6 00	30.500	3-10.00	9.00	o°
RC. Bridge	33+973	30.578	3-10.00	9.00	30°
RC, Bridge	36+600	40.500	4-10.00	9.00	o°
RC. Bridge	41+357	30.500	3-10.00	9.00	o°.

Costs estimated to be 6,084,843 Baht from the bills of quantities.

- 23) Miscellaneous costs are 1,008,200 Baht based on a proportion of total.
- 24) Overheads: zero.

Section 4 (sta. 41+500 to sta. 68+400 = 26.9 km)

- 1) Horizontal alignment: 6.32 degree/km measured from the construction plans.
- 2) As for Section 1.
- 3) Geometric design standards: as for Section 3, except design speed of 65 km/hr.
- 4) As for Section 1.
- 5) As for Section 2.
- 6-13) As for Section 1.
- 14) As for Section 2.

15-17) As for Section 1.

- 18) Average length of pipe is 21.89 meters from construction plans.
- 19) As for Section 1.
- 20) No cross-flow drainage.

21) Minor river crossings at:

Station	Catchment area, ha	Terrain run-off coeff.
42+000	25.0	0.8
44+260	57.0	0.8
45+300	20.0	0.8
45+850	20.0	0.8
46+275	9.0	0.8
46+900	37.0	0.8
51+135	20.0	0.8
51+550	20.0	8.0
52+855	45.0	0.5

22) Major river crossings at:

	Station	Length	Spans	Width	Skew	
RC. Bridge	43 +152	30,508	3-10.00	9.00	10°	
RC. Bridge	49+140	24.532	3-8.00	9.00	20°	
RC. Bridge	50+719	24.500	3-8.00	9.00	o°	
RC. Bridge	53+200	18.500	3-6.00	9.00	20°	
RC. Bridge	54+7 40	21.500	3-7.00	9.00	20°	
RC. Bridge	55+577	40,500	4-10.00	9.00	10°.	
RC. Bridge	56+863	40.500	4-10.00	9.00	30°	
RC. Bridge	58+291	40.500	4-10.00	9.00	20°	
RC. Box culvert	59+242	width = 8.30	; 3 boxes 2.	50×2.00; le	ength = 15.30; skey	w=0°
RC. Bridge	62+045	40.500	4-10.00	9.00	o°	
RC . Bridge	64+594	30.500	3-10.00	9.00	o°	

536,934

					158	
	Station	Length	Spans	Width	Skew	
RC. Bridge	66+500	40.500	4-10.00	9.00	ર્જ	
RC. Box culvert	68+080 v	vidth = 8.3); 3 boxes 2.5	50x2.00; le	ngth = 14.4; sl	kew=0
Costs estimat	ed to be 6	,25 0 ,73 5 B	aht from the	bills of qua	ntities.	
23) Miscellaneo	us costs are	1,094,92	5 Baht based	on a propor	tion of total.	
24) Overheads:	zero.					
D						
Details of Struct	ures Costs,	Baht				
Furnish and dr	ive concret	e test pile			17,120	
Furnish and dr	ive concret	e piles			409,880	
Test loading c	oncrete pile	es			54,000	
Concrete for s	tructures C	lass "A" bri	idges, box cu	lverts 6	,488,270	
Reinforcement	for bridges	and box cu	lverts	3	140,975	
Railing, reinfo	rced concre	ete		* 21	55,050	
Granular back	fill materia	1			177,021	
Channel and s	tructural ex	cavated ea	rth		624,494	
Channel and si	tructural ex	cavated ro	ck		50,900	
Grouted riprap			Addition .	1	,192,440	
Other costs of	culverts			1	,759,980	
Total costs				13	,970,130	
These costs obt	ained from	the bills o	of quantities o	and final rep	oort.	
Miscellaneous Co	sts, Baht	¥				
Removal of exi	sting struct	ures			112,300	
Alterations to	Pasak River	Bridge			140,000	

Concrete curb, gutter and sidewalk (Section 1)

Right-of-way monuments	40,880
Delineators	79,110
Bench marks	5,800
Kilometer posts	25,160
Furnish and erect road signs	12,060
Furnish and install sign posts	50,160
Highway stripping, reflectorized	531,523
Marking at bridge ends	23,700
Furnish and install settlement platforms	1,200
Overhead charge for under-runs	234,071
Force account	23,350
Contract change orders	1,590,899
Total costs, obtained from the final report	3,407,147

Table D 2 Vehicle Operating Costs for Saraburi - Lomsak Highway

- 1) Interest rate on capital used to buy new vehicles is 12 percent per annum.
- 2) Fuel prices: the economic cost of fuel was based on Road User Costs in Thailand-1973 by T.P.O'Sullivan & Partners, and gave petrol at 1.20 Baht per liter and diesel fuel at 0.90 Baht per liter. Lubricants were taken at 7.10 Baht per liter.
- 3) Vehicle maintenance labour rates were taken from average wage of a mechanic, includes the materials, machinery, overheads, and profit associated with labour in the garage business and found to be 32.80 Baht per hour (T.P.O'Sullivan & Partners, 1973)

- 4) 6 vehicle types were considered in the present study: (1) cars and taxis;
 (2) light trucks; (3) light buses; (4) heavy buses; (5) 2-axle heavy trucks;
 and (6) 3-axle heavy truck.
- 5) For heavy trucks: information on unladen and laden weight and equivalence factors were obtained from 4-day axle-load survey. The results are given in Table E1 and Table E2.
- 6) For each vehicle class, average new vehicle prices and replacement tyre prices were obtained from <u>Road User Costs in Thailand - 1973</u>. These are given in Table E3.
- 7) Details of operation of the vehicle were obtained largely from Road User

 Costs in Thailand-1973. The values used are given in Table E4.
- 8) Figures for average daily traffic in the two directions were obtained from Traffic Volumes and Flow Maps; Planning Division, Highway Department.

Table D3 Road Maintenance Data for Saraburi - Lomsak Highway

Plant hire rates (Baht per hour): these rates were obtained from Mechanical

Division, Highway Department

4500 - liter bitumen distributor	110.0
0.25 - tonne vibrating roller	20.0
Motor grader	110.0
10 - tonne roller	45.0
Tractor mower	50.0
Water tanker	67.5
Tipper truck	67.5

Labour rates (Baht per hour): these rates were obtained from Economic

Feasibility Study (DE LEUW, CATHER, 1964)

Labourers	4.0
Truck drivers	5.0
Plant operators	8.0
Foremen	10.0

Material costs: these costs were obtained from Final Report (De Leuw, Cather,

1970) and Rural Road Project (Ministry of Interior, 1971)

Ditumen	1.5 Baht per liter
Surface dressing stone	132.0 Baht per m ³
Base patch material	132.0 Baht per m ³
Surface patch mix	35 0.0 Baht per m ³
Diesel fuel	1.0 Baht per liter
Transport distances:	
Bitumen	20 km
Surface dressing stone	20 km
Depot to site	20 km

Table E 1 Heavy Trucks by Weight

Gross					eavy Truc			
Weight, tonnes	Saraburi - Ban Phu Kae SB NB				Ban Phu Kae - Lamnarai SB NB			
	Province and the second	177	2-axle		-			
5.5 - 6.5			8				11'	
6.5 - 7.5	2		1	110	1			52
7.5 - 8.5	1		5					
8.5 - 9.5			2					
9.5 - 10.5	10		7		8		1	
10.5 - 11.5	11		2		3			
11,5 - 12,5	2		1	1	1		1	1
12.5 - 13.5				1				
13.5 - 14.5				1				
14.5 - 15.5		1		2				2
15.5 - 16.5		3		1		2		1
16.5 - 17.5		6		8		3		4
17.5 - 18.5	1 5	85		11		39		1
18.5 - 19.5		4		1		3		
19.5 - 20.5		37		1		12		1
20,5 - 21,5		1				3		

Table E2 Heavy Truck Descriptions

Item		Saraburi -	Ban Phu	Kae	Ban Phu Kae-Lam narai		
		Heavy bus	2 - axle	3-axle	Heavy bus	2-axle	3-axle
Brake horse power Unladen weight, tonnes		185 7.0	125 5.5	125 7.2	185 7.0	125 5.5	125 7.2
Sourhbound	Equivalence factor	0,23	0.45	1.32	0.23	0.41	1.27
Direction	Load, tonnes	2.0	2.6	1.9	2.0	0.8	1.5
Northbound	Equivalence factor	0.23	0.22	0.20	0,23	0.11	0.15

Table E3 Vehicle and Tyre Prices, (Economic Costs), Baht

Vehicle Type	New Vehicle Price	Replacement Tyre Price		
Cars and taxis	62,000	1,400		
Light trucks	58,000	2,200		
Light buses	62,000	2,200		
Heavy buses	330,000	8,200		
2 - axle trucks	168,500	8,200		
3 - axle trucks	194,200	13,700		

Table E4 Vehicle Crews, Usage, Passengers' Time and Overheads

Vehicle Type	Number of crew	Crew cost per hour	Avarannual crew hour	Passenger's time B/hr	Av, annual kilometers	Standing cost, %
Cars and taxis	0	-	0	30.0	18,000	10
Light trucks	1	6.0	1,500	10.0	25,000	25
Light buses	1	6.0	2,500	20.0	35,000	25
Heavy buses	2	10.0	5,500	50.0	70,000	25
2 - axle trucks	1	6,0	5,500	7.0	70,000	25
3 – axle trucks	1	6.0	5,500	7.0	70,000	25

Table E 5 Traffic Flows on Saraburi - Lomsak Highway in 1970

(By Exponential Regression)

Vehicle type	Average Daily Traffic							
	Section 1	Section 2	Section 3	Section 4				
Cars and taxis	1,590	795	273	273				
Light trucks	448	224	164	164				
Light buses	367	183	118	118				
Heavy buses	448	224	107	107				
2 – axle trucks	7 00	350	200	200				
3 - axle trucks	1,209	605	370	370				
Total	4,762	2,381	1,232	1,232				

¹Two lanes, only Southbound direction, taken as half of ADT on Section 1.

VITA

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